

## JAYPEE UNIVERSITY OF INFORMATION TECHNOLOGY, WAKNAGHAT

## MID SEMESTER EXAMINATION-2015

## B.Tech VI Semester

COURSE CODE: 10B11CE611

MAX. MARKS: 30

COURSE NAME: Design of Steel Structures

COURSE CREDITS: 04

MAX. TIME: 2 HRS

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*Note: (i) All questions are compulsory.*

*(ii) For numerical problems write in detail all the steps needed for the solution.*

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## Section A

(Marks: 6)

1. Of the drilled and punched bolt holes which one is preferred and why?
2. For joints subjected to fatigue, which type of connector is preferred and why?
3. Sketch the connection details of two angles connected on the same side of gusset plate. Each angle is connected to the gusset plate by single row of 16 diameter bolts three in numbers.
4. Calculate maximum pitch and maximum edge distance for 16mm diameter bolt to connect 80ISF12 and 80ISF10 in tension. Steel is of grade Fe410.
5. What is the minimum and maximum size of the fillet weld for following cases:
  - a. If ISA 75 X 75 X 8mm is connected to gusset plate with rolled edge through fillet weld along the longitudinal axis.
  - b. If two plates 6mm thick are to be lap joined by fillet weld.
6. What is the effective throat thickness for groove weld for following case:
  - a. If a single groove weld is provided to connect two plate thickness of 18mm and 16mm.
  - b. If Double groove weld is provided to connect two plate thickness of 18mm and 16mm.

### Section B

(Marks: 9)

- (a) Explain the phenomenon of load transfer in high strength friction grip bolts.  
(b) Prove that size of weld for fillet weld is proportional to thickness of plates
- Fig. Q2 shows a hanger connection carrying a factored load of 250kN. Investigate the safety for the two 30mm diameter bolt of grade 5.6. Grade of steel is Fe415

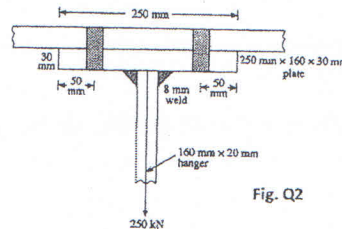


Fig. Q2

- Give Reasons:

- Why there is a separate provision for maximum pitch in bolted connection for member subjected to Tension and Compression in IS 800:2007
- Why correction factor on shear strength of bolt is done if the grip length is large.
- Why minimum size of the weld is recommended in IS 800: 2007

### Section C

(Marks: 15)

- The lower chord joint of a roof truss is with a continuous chord member as shown in Fig Q1. Design and detail the joint using M20 bolts of the product grade C and the property class 4.6. Steel property is  $f_y=250\text{MPa}$  and  $f_u=410\text{MPa}$ .

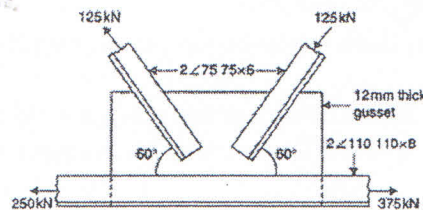


Fig. Q1

- For Q1. Design and detail the welded connection.
- Two plates with cross sections of 125 X 8mm and 125 x 12mm which are subjected to a tension of 100kN at working load are to be connected by lap joint.
  - Determine the sizes of end fillets to connect the two plates.
  - Detail 12mm diameter 5.6 grade bolt arrangement to connect the two plates.
 The ultimate strength of the plate is,  $f_u = 410\text{MPa}$ .

10.4.7 Where prying force,  $Q$  as illustrated in Fig. 16 is significant, it shall be calculated as given below and added to the tension in the bolt.

$$Q = \frac{l_v}{2l_c} \left[ T_b - \frac{\beta \eta f_o b_e t^2}{27 l_c l_v^2} \right]$$

where

$l_v$  = distance from the bolt centreline to the toe of the fillet weld or to half the root radius for a rolled section,

$l_c$  = distance between prying force and bolt centreline and is the minimum of either the end distance or the value given by:

$$l_c = 1.1r \sqrt{\frac{\beta f_o}{f_y}}$$

where

$\beta$  = 2 for non pre-tensioned bolt and 1 for pre-tensioned bolt,

$\eta$  = 1.5,

$b_e$  = effective width of flange per pair of bolts,

$f_o$  = proof stress in consistent units, and

$t$  = thickness of the end plate.

### 10.3.5 Tension Capacity

A bolt subjected to a factored tensile force,  $T_b$ , shall satisfy:

$$T_b \leq T_{db}$$

where

$$T_{db} = T_{nb} / \gamma_{mb}$$

$T_{nb}$  = nominal tensile capacity of the bolt, calculated as:

$$0.90 f_{ub} A_n < f_{yb} A_{sh} (\gamma_{mb} / \gamma_{mb})$$

where

$f_{ub}$  = ultimate tensile stress of the bolt,

$f_{yb}$  = yield stress of the bolt,

$A_n$  = net tensile stress area as specified in the appropriate Indian Standard (for bolts where the tensile stress area is not defined,  $A_n$  shall be taken as the area at the bottom of the threads), and

$A_{sh}$  = shank area of the bolt.

## 6.2 Design Strength Due to Yielding of Gross Section

The design strength of members under axial tension,  $T_{dg}$ , as governed by yielding of gross section, is given by

$$T_{dg} = A_g f_y / \gamma_{m0}$$

where

$f_y$  = yield stress of the material,

$A_g$  = gross area of cross-section, and

$\gamma_{m0}$  = partial safety factor for failure in tension by yielding (see Table 5).

## 6.3 Design Strength Due to Rupture of Critical Section

### 6.3.1 Plates

The design strength in tension of a plate,  $T_{dt}$ , as governed by rupture of net cross-sectional area,  $A_n$ , at the holes is given by

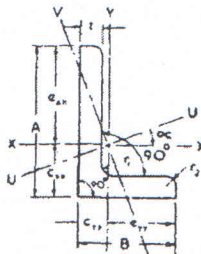
$$T_{dt} = 0.9 A_n f_u / \gamma_{m1}$$

where

$\gamma_{m1}$  = partial safety factor for failure at ultimate stress (see Table 5),

$f_u$  = ultimate stress of the material, and

$A_n$  = net effective area of the member given by,



Angle Section	exx	Cxx	Cyy	eyy
ISA 75 x 75 x 6	54.4mm	20.6mm	20.6mm	54.4mm
ISA 110 x 110 x 10	79.2mm	30.8mm	30.8mm	79.2mm

### 10.3.3 Shear Capacity of Bolt

The design strength of the bolt,  $V_{dsb}$ , as governed shear strength is given by:

$$V_{dsb} = V_{nsb} / \gamma_{mb}$$

where

$V_{nsb}$  = nominal shear capacity of a bolt, calculated as follows:

$$V_{nsb} = \frac{f_u}{\sqrt{3}} (n_s A_{sh} + n_p A_{nb})$$

where

$f_u$  = ultimate tensile strength of a bolt;

$n_p$  = number of shear planes with threads intercepting the shear plane;

$n_s$  = number of shear planes without threads intercepting the shear plane;

$A_{sh}$  = nominal plain shank area of the bolt; and

$A_{nb}$  = net shear area of the bolt at threads, may be taken as the area corresponding to root diameter at the thread.

### 10.3.4 Bearing Capacity of the Bolt

The design bearing strength of a bolt on any plate,  $V_{dpb}$ , as governed by bearing is given by:

$$V_{dpb} = V_{npb} / \gamma_{mb}$$

where

$$V_{npb} = \text{nominal bearing strength of a bolt} \\ = 2.5 k_b d t f_u$$

where

$$k_b \text{ is smaller of } \frac{e}{3d_o}, \frac{p}{3d_o} - 0.25, \frac{f_{ub}}{f_u}, 1.0;$$

$e, p$  = end and pitch distances of the fastener along bearing direction;

$d_o$  = diameter of the hole;

$f_{ub}, f_u$  = ultimate tensile stress of the bolt and the ultimate tensile stress of the plate, respectively;

$d$  = nominal diameter of the bolt; and

$t$  = summation of the thicknesses of the connected plates experiencing bearing stress in the same direction, or if the bolts are countersunk, the thickness of the plate minus one half of the depth of countersinking.